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Criteria for the selection and design of joints for street pavements in natural stone

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Corresponding Author: Professor Felice Giuliani, M.D., Full Professor

Corresponding Author's Institution: University of Parma

First Author: FEDERICO AUTELITANO, Ph.D.

Order of Authors: FEDERICO AUTELITANO, Ph.D.; ERIKA GARILLI, Ph.D.; Felice Giuliani, M.D., Full Professor

Abstract: Natural stone pavement can produce a high quality attractive streetscape with a long service life. Although the formal analogy with the interlocking concrete pavements, the peculiarities and singularities of the stone elements pose enormous challenges to the designers which have at their disposal only limited standards and guidelines. The joints represent one of the most important part of the stone paving system, performing multiple tasks: stabilize the elements, transmit and dissipate loads, control the water disposal and contribute to the aesthetic. But, the analytical study of the joints is inexplicably an understudied topic in the technical and scientific literature: many problems resulted in stone pavements from lack of understanding of joints' purposes and functions. Thus, the paper highlighted the conceptual steps necessary to the selection of the optimal jointing solution. Several aspects related to the stone elements' characteristics and laying that contribute to the joints' optimized performance have been evaluated. After a brief description of the main defects that can affect stone pavements, a state of practice regarding the jointing materials with the related application techniques have been reported.

COVER LETTER FOR SUBMISSION OF MANUSCRIPT

Parma, 14th May 2020

Dear Editor-in-Chief,

We have received your letter indicating that a revised version of our manuscript “CRITERIA FOR THE SELECTION AND DESIGN OF JOINTS FOR STREET PAVEMENTS IN NATURAL STONE” (CONBUILDMAT-D-20-02803) was requested. We strove to provide you a new better revised article following closely all the reviewers’ comments, which were particularly useful for identifying aspects not well clarified and for meeting Journal standards. From that perspective, we would like to thank them for their valid opinions and time spent.

With the resubmission of this manuscript I would like to undertake that:

- I will be serving as the corresponding author for this manuscript and I have assumed responsibility for keeping my coauthors informed of our progress through the editorial review process, the content of the reviews, and any revisions made;
- This manuscript represents an original work;
- The contents of this manuscript have not been copyrighted or published previously;
- The contents of this manuscript are not now under consideration for publication elsewhere;
- All authors of this research have contributed substantially to the manuscript;
- All authors of this paper have read and approved the final version submitted;

- All previously published work cited in the manuscript has been fully acknowledged
- There is no type of real or perceived conflicts of interest

Please let me know of your decision at your earliest convenience.

Sincerely,

Felice Giuliani

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1 **Title**

2 **Criteria for the selection and design of joints for street pavements in natural stone**

3

4 **Author names and affiliations**

5 Federico Autelitano^a, Erika Garilli^a and Felice Giuliani^{a*}

6 *^aDipartimento di Ingegneria e Architettura, University of Parma, Parma, Italy.*

7 * Dipartimento di Ingegneria e Architettura, University of Parma, Parco Area delle
8 Scienze, 181/A - 43124 Parma, Italy. E-mail: felice.giuliani@unipr.it Phone :+39 0521
9 905905

11 **Corresponding author**

12 Felice Giuliani

13 E-mail: felice.giuliani@unipr.it Phone :+39 0521 905905

14 Dipartimento di Ingegneria e Architettura, University of Parma, Parco Area delle
15 Scienze, 181/A - 43124 Parma, Italy

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17 **Criteria for the selection and design of joints for street pavements in**
18 **natural stone**

19 **Abstract**

20 Natural stone pavement can produce a high quality attractive streetscape with a long
21 service life. Although the formal analogy with the interlocking concrete pavements, the
22 peculiarities and singularities of the stone elements pose enormous challenges to the
23 designers which have at their disposal only limited standards and guidelines. The joints
24 represent one of the most important part of the stone paving system, performing
25 multiple tasks: stabilize the elements, transmit and dissipate loads, control the water
26 disposal and contribute to the aesthetic. But, the analytical study of the joints is
27 inexplicably an understudied topic in the technical and scientific literature: many
28 problems resulted in stone pavements from lack of understanding of joints' purposes
29 and functions. Thus, the paper highlighted the conceptual steps necessary to the
30 selection of the optimal jointing solution. Several aspects related to the stone elements'
31 characteristics and laying that contribute to the joints' optimized performance have been
32 evaluated. After a brief description of the main defects that can affect stone pavements,
33 a state of practice regarding the jointing materials with the related application
34 techniques have been reported.

35
36 **Keywords:** stone pavement; joint filling; restoration; polymeric sand; resin; cement
37 mortar.

38

39 **Introduction**

40 The excellent technical-performance characteristics, together with the flexibility of the
41 size and the inherent characteristics capable of determining colors, textures and unique
42 aesthetic results have made the natural stone the par excellence paving material since
43 the Roman times [1,2]. After a long period spent covering the authentic historic
44 pavements with asphalt, we are now witnessing a renewed interest in stone pavements
45 not only for the necessary conservation, maintenance and enhancement of this
46 impressive artistic, archaeological and cultural heritage, but just as current reuse in a
47 modern sense of technologies often wrongly considered obsolete and no longer meet the
48 today's infrastructure needs (Fig.1) [3,4]. Stone pavements belong to the broader family
49 of the elemental or segmental pavements, i.e. pavements in which the surface course is
50 made up of individual units placed close together to each other and embedded in a
51 bound or unbound bedding layer or laying course [5]. Although the formal analogy with
52 the interlocking concrete pavements, whose use has been well researched and supported
53 by an established know-how [6-8], the peculiarities and singularities of the stone
54 elements pose enormous challenges to the designers which have at their disposal only
55 very limited national standards and guidelines. The only international harmonized
56 standards concern the inherent mechanical and physical-chemical performance
57 requirements of stone units. Thus, the modern study of these pavements, according to a
58 technical engineering perspective, for the project of a new surface or the conservation
59 and restoration of a historic one requires a specific design of the laying conditions with
60 maximum attention to the compatibility and the interaction between the stone elements,
61 the bedding layer and the joints, i.e. the gaps or the distances between adjacent paving
62 units, or units and restraints or perimeters. The joints represent one of the most
63 important part of the composite stone paving system, performing multiple tasks:

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stabilize the elements, transmit and dissipate loads, control the water disposal and contribute to the aesthetic quality of the paving [9,10]. Several jointing solutions have been traditionally used and each one responds to specific needs with its advantages and disadvantages [11,12]. However, talking about joints, it is not enough simply to consider the material that fills the gaps between units, but represent complex systems that affect with their characteristics the behavioral mechanism of the whole pavement and have a decisive influence on the selection of the stone elements' size and dressing and on the laying techniques and patterns. But, the analytical study of the joint is inexplicably an underestimated and understudied topic in the technical and scientific literature: many problems resulted in stone pavements from lack of understanding of joints' purposes and functions.



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Figure 1 - Construction of a stone pavement in urban context

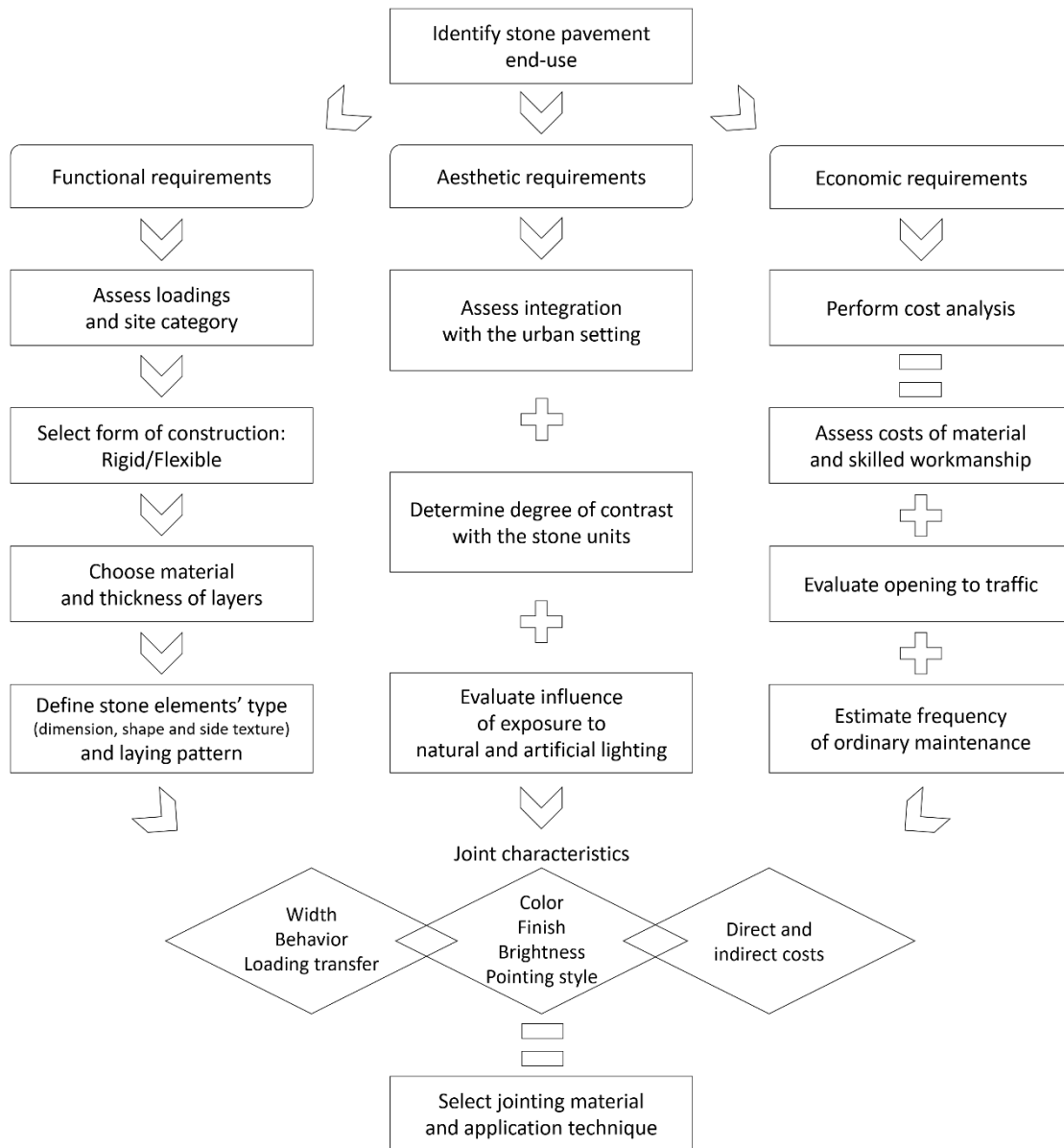
Thus, the paper aims to highlight the conceptual steps that can allow a rational choice of the optimal jointing solution. Several aspects related to the characteristics of the stone elements (size, shape, side-texture) and laying that contribute to an optimized performance of the joint have been analyzed in detail. After a brief description of the

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81 main types of defects that can be caused by mechanical and physical-chemical stresses
82 but also by poor or inaccurate design, installation (workmanship) and maintenance, a
83 state of practice regarding the current jointing materials with the related application
84 techniques have been mentioned. Also jointing solutions which were in common use in
85 the XIX century and have partially fallen into disuse have also been included, because
86 in maintenance or restoration works, according to conservation principles, original
87 materials including details and finishing and traditional skills have to be used,
88 eventually supported by modern techniques only where traditional methods may prove
89 inadequate.

90 **Design criteria**

91 The choice of the best jointing solution should come from a multi-criteria analysis, in
92 which the designer comes up with a decision downstream of an analytical process based
93 on functional, aesthetic and **economic** hierarchical requirements (Fig. 2).



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Figure 2 - Multi-criteria analytical process for selecting the best jointing solution

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The definition of these requirements mainly depends on the pavement end-use, the construction technique and the stone units' material and type (dimension, shape and side texture). The broad concept of pavement end-use considers aspects related to the induced loads, the type of prevailing user (pedestrian, cyclist, vehicle) and the area in which the pavement is inserted (private, residential, historical, park). The system of stresses to which the pavement should be able to resist, and therefore have to be taken

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115 into account during the design phase, mainly concerns the presence of static or dynamic
 116 loads transmitted by pedestrian or vehicular traffic [13]. On the basis of the magnitude
 117 and frequency of this loading condition, a number of “site categories” or
 118 “operation/stress classes” can be identified. Stone surfaces are also strongly affected by
 119 secondary loadings, i.e. stresses induced by the layout and geometry of a particular site
 120 which have the effect of concentrating the loading (gradients, junctions, restricted road
 121 width, channelization, drainage channels) [9]. Besides mechanical loadings, the effects
 122 of hygral-thermal conditions, linked to climatic and seasonal variations (UV radiation,
 123 high/low temperature, frost, snow, rain), or caused by the aggression of chemical
 124 substances (air pollution, salts, lubricants or fuels) should be taken into account [14].
 125 The designer has the possibility to select either a flexible (Fig. 3a) or rigid (Fig. 3b)
 126 pavement construction, that obviously underlies a different deformation behavior under
 127 loads, within the limitation of their bearing capacity [15].

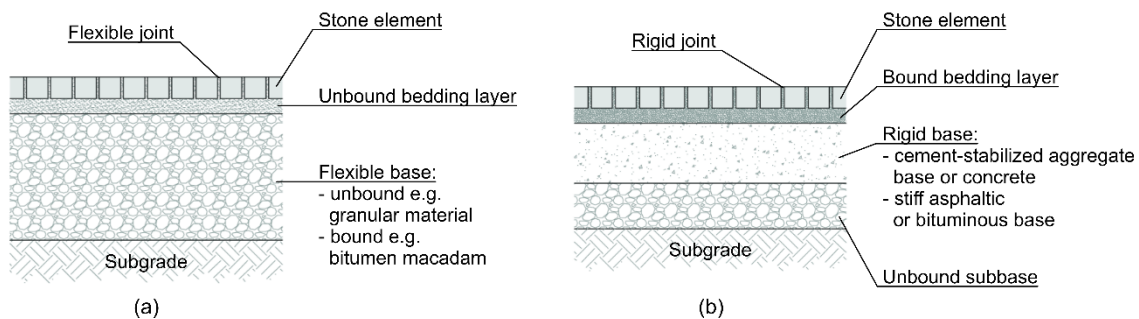


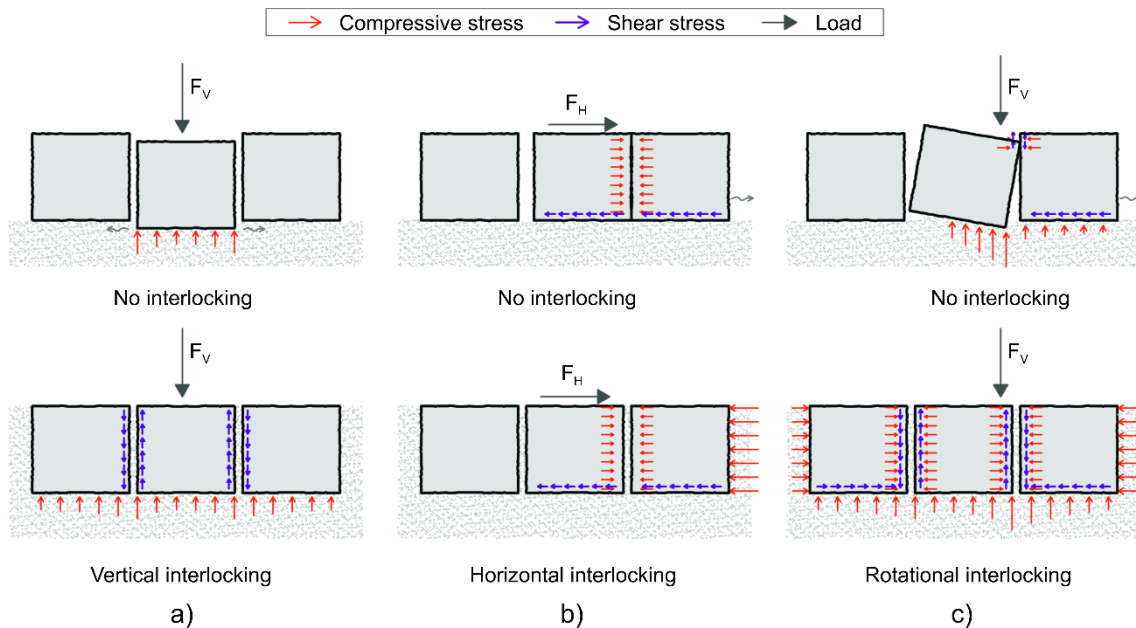
Figure 3 - Flexible (a) and rigid (b) form of construction

130 In addition to the type of materials that constitute the layers, also features of stone units,
 131 their laying patterns, and joint width determine the way in which the surface course
 132 responds under load, influencing the pavement structural capacity [16]. Stone elements
 133 come in a wide variety of materials, dimensions and shape, ranging from rounded
 134 cobbles, to parallelepiped shaped deep elements (cubes, setts, cut blocks) and to shallow
 135 and large slabs (flagstones and polygonal flags) [17,18]. In general terms, shallower
 136 units respond in a flexible manner to loads and are best suited to flexible construction;

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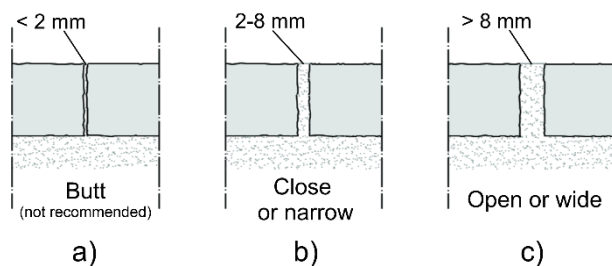
137 as the depth of the units increases the behavior become stiffer. Typically, surfaces that
138 react flexibly rely more on the laying course than on the joints to sustain loading, while
139 rigid stone pavements are stabilized by the setting action of the joint material, which
140 provide at the same time the primary resistance to load [9]. In accordance to the
141 principle of execution consistency, the compatibility between bedding and jointing
142 materials is required: flexible form of construction uses unbound bedding and flexible
143 jointing (unbound or bound); whereas rigid pavement uses a rigid bound material in
144 both the laying course and the joints [10].

145 In site categories subjected to regular vehicular traffic, eventually receiving heavy
146 vehicles, the joint is required to contribute to the structural performance of the whole
147 pavement. The joint performs its structural function in a different way when a
148 flexible/rigid or unbound/bound material is used. Flexible joints are able to
149 accommodate any small relative movements of the elements, induced by traffic,
150 temperature variations or tree root growth, without failing [15,16,19]. Rigid materials
151 are instead characterized by higher compressive and shear strengths, but are prone to
152 local cracking even after a single stress application in excess of their capacity [20,21].
153 Unbound materials develop mechanical interlock, also called interlocking, and friction
154 to “lock” the elements and transmit loads, whereas bound products perform their
155 functions exploiting the bond adhesion strength between the stone and the joint material
156 [22]. Interlocking, which is expressed in its three types vertical (resistance to sinking),
157 horizontal (resistance to shifting) and rotational (resistance to tipping), represents the
158 inability of a stone element to move independently from its neighbors (Fig. 4).



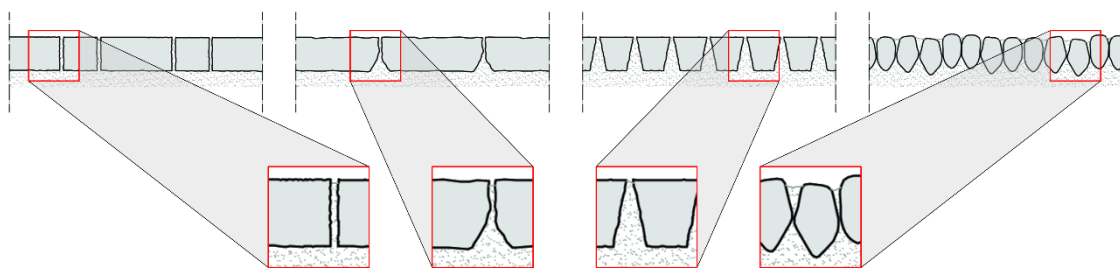
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160 **Figure 4 - Interlocking mechanisms: vertical (a), horizontal (b) and rotational (c)**

161 Specifically, vertical interlock is primary achieved by shear transfer of loads to
 162 surrounding units through the joints (Fig. 4a), while the horizontal one through the use
 163 of laying patterns that disperse forces from braking, turning and accelerating vehicles
 164 (importance of edge restraint) (Fig. 4b). Rotational interlock is maintained by the
 165 element being of sufficient thickness, placed closely together (joint width), and
 166 restrained by a curb from lateral forces (Fig. 4c) [8]. The stone side-wall texture and the
 167 joint width play an important role in the development of interlocking. Sawn, split,
 168 cropped or sawn and re-textured units' sides produce very different adhesion and
 169 frictional bonds with the jointing material [23,24]. In terms of joint width, three main
 170 types can be defined: butt (adjacent paving units are in direct contact with one another),
 171 close or narrow (2-8 mm) and open or wide (> 8 mm) (Fig. 4).



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173 **Figure 5 - Joint width types: butt (a), close or narrow (b) and open or wide (c)**

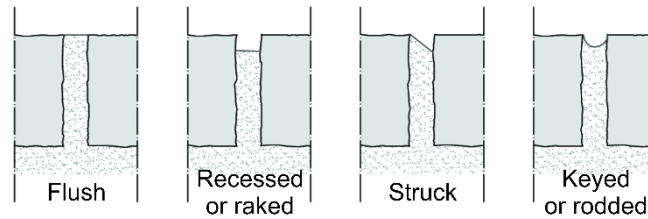
174 In the first case the risk of damage to the edge or arris of the paving units (spalling or
175 chipping) is extremely high (Fig. 5a). Increasing too much the width, it reduces the
176 possibility of obtaining a satisfactory vertical and rotational interlock, especially if an
177 unbound material is used: open joints require a bound product for best result (Fig. 5c).
178 In wide open joints, a loss of filling makes impossible to partially transfer any stress
179 induced by traffic loads to the adjacent elements. A consistent joint width in the surface
180 over the whole depth and length of the joint is a fundamental design parameter to ensure
181 a completely filling and a homogeneous compaction of the material. But, the
182 achievement of this requirement is a very difficult practice using natural stone elements,
183 above all with round or square but tapered units (Fig. 6).



184
185 **Figure 6 - Inconsistent design joint width**

186 Obviously, the joint width is an important factor with regards to the pavement drainage.
187 Proper and effective drainage is a fundamental design parameter to consider in order to
188 ensure both the users' safety and the structural long term performance of the pavement.
189 Depending on the type of support layer and the techniques and methods of drainage
190 (surface or sub-surface), the joint may be required to be impervious or completely
191 permeable. Where frost action is likely to occur, above all in climates with cold winters,
192 a pervious jointing material is a preferred solution [11,25].
193 However, most of the stone pavements are located in historical centers and in
194 monumental contexts, shaping the architectural and spatial character of streets or
195 squares and redrawing the urban landscape [26,27]. Thus, the often overlooked aesthetic

196 requirements are essential elements and very often priority with respect to the functional
197 ones. The joint itself becomes a decorative motif that can make a decisive contribution
198 to the overall pattern of the pavement. The material, color and detail of a jointing
199 material can have a dramatic effect on the finished appearance of the work, completely
200 changing its character in some cases [28] (Fig. 7).



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Figure 7 - Pointing styles

203 The chosen solution should never detract from the stone by being more visually
204 dominant. Strap-pointing with raised bands covers the edges and causes the pointing to
205 be too intrusive. Thus, slightly recessed joints allow the edges to be seen, but they have
206 the disadvantage of catching dirt and allowing the accumulation of water which may
207 make them unacceptable in some circumstances [29]. A good jointing material should
208 complement the stone, offering a suitable degree of contrast that emphasizes the
209 individual element: an exact match, a strong contrast, or a color that blends with units
210 represent options to be assessed at the design stage, considering the natural (direct
211 sunlight or shaded area) and artificial lighting (color temperature and color rendering
212 index) conditions and exposure [30]. The chromatic effects can be obtained both by
213 exploiting the natural color of the jointing medium both through the use of pigments,
214 possibly characterized by enhanced properties, such as photoluminescence or
215 thermochromism, for special and scenic applications [31-33]. Besides, the joint color is
216 strongly affected by finishing and detail parameters (smooth vs grainy and glossy vs
217 matte), which influence the light reflection.

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218 But, the final decision cannot be separated from a detailed cost analysis that takes into
219 account all the direct and indirect costs associated with the joint project, which
220 obviously are not limited to the purchase of jointing materials. Costs also include the
221 labor expenses for applications and repair/maintenance: this cost item can be extremely
222 variable. Some jointing materials require advanced special-purpose tools, skilled and
223 experienced operators and high frequency of ordinary maintenance, i.e. aspects that
224 considerably raise the total funds needed to complete the project and correctly maintain
225 the pavement.

226 **Features of elements**

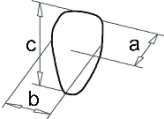
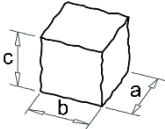
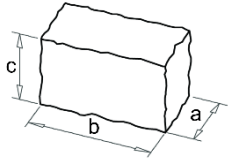
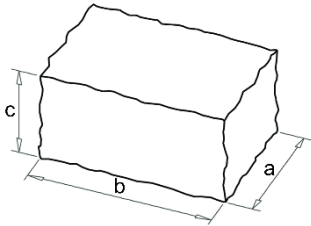
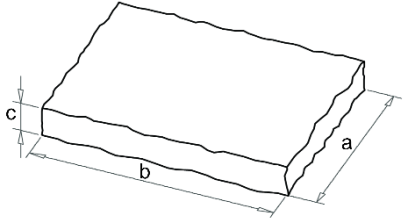
227 Several inherent features of stone elements (petrographic nature, mechanical and
228 physical-chemical performances) and others resulting from quarrying, cutting and
229 dressing process (size, shape and face texture) strongly influence the pavement design,
230 including the selection of the joint solution and width [12,34].

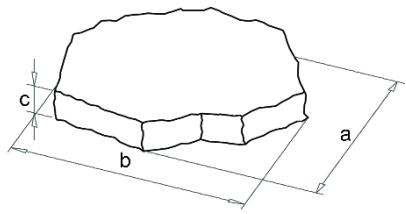
231 The materials used for stone pavements greatly varied from place to place; the choice
232 was dictated, especially for the pre-XIX century projects, by logistical constraints
233 related to distance and ease of material procurement, while today due to the market
234 globalization, designers can select different petrographic varieties characterized by
235 peculiar properties. The most commonly used materials are porphyry, granite (and
236 similar lithotypes), basalt, sandstone, trap and compact limestone [18,35]. Thus, jointing
237 material must be firstly chemically compatible with the stone and with the surface
238 finishing, particularly when sanding, brushing or bush-hammering on site is planned.

239 The dimensional characteristics of stone elements are an important morphological
240 variable, conditioning the aesthetic rendering, the pavement behavior and the joint
241 network. Stone pavements use units characterized by a multiplicity of shapes, more or

242 less regular, and with size that vary from a few centimeters (small cubes) to over one
 243 meter in length (giant flags) (Tab. 1).

244 **Tab.1 - Stone elements used for paving**

	Dimensions [mm]	Joint width [mm]	Typical application
Cobblestone			
	$a = 60 \div 80$ $b = 80 \div 120$ $c = 120 \div 160$	$5 \div 30$ (surface)	Pedestrian areas in historic settings (footpaths, squares, arcades)
Cube			
	$a = 40 \div 200$ $b \approx a$ $c \approx a$	$6 \div 10$ $(40 < a < 60)$ $8 \div 12$ $(70 < a < 140)$ $10 \div 15$ $(150 < a < 200)$	Pedestrian areas Pedestrian areas with occasional vehicular traffic Roads Footways overridden by vehicular traffic
Sett			
	$a = 70 \div 150$ $b \approx 2a$ $c = 100 \div 180$	$8 \div 12$ $(100 < c < 120)$ $10 \div 15$ $(150 < c < 180)$	Roads Car parks with occasional heavy traffic Footways overridden by vehicular traffic
Block			
	$a = 150 \div 250$ $b \approx 2a$ $c = 100 \div 200$	$10 \div 12$ (surface) Max. $22 \div 25$ (any part)	Street and roads
Flagstone			
	$a = 200 \div 500$ $b = 300 \div 500$ $c = 50 \div 100$	$6 \div 12$ Min. 4 (sawn and shallow units)	Pedestrian areas (footpaths, squares, arcades), with occasional vehicular traffic
Polygonal flagstone			



$$a = 200 \div 500$$

$$b = 300 \div 500$$

$$c = 50 \div 100$$

< 30

Private drives, paths, patio
and hard landscaping
projects

245

246 Quadrangular elements, both rectangular and square, are the most commonly used.

247 Unlike industrial products, which are strictly controlled in tolerances and modularity,

248 slight deviations from the standard dimensions of stone paving elements are tolerable

249 [17,36]. In addition to calibrate the density of the pattern in a progressive saturation of

250 the basic shape, the size also responds to the functional needs of a pedestrian or

251 vehicular user. However, horse-drawn carriages were the reference user in the XIX

252 century. Thus, requirements for units' length and width were reported in specifications

253 of that time, on the basis of horseshoes' dimension, stride length and number of

254 longitudinal and transverse joints, which offered the animals a better foothold,

255 preventing from dangerous slipping [25,37]. The mainly two-dimensional characteristic

256 of the elements must not make us forget the importance of the depth (or thickness),

257 which influences the expressive result of the surface in a lesser way, but can limit their

258 application in certain site categories and conditions the pavement behavior under

259 loading. Generally, deep elements ($c > 100 \text{ mm}$) are predominantly stabilized by

260 jointing and can be used also in medium-high intensity traffic applications, whereas

261 shallow units ($50 \text{ mm} < c < 100 \text{ mm}$) are stabilized by bedding and are destined to

262 pedestrian projects subjected to occasional overrun of vehicles (Tab. 1). In addition to

263 the size and shape, also dressing degree and side-wall texture affect the role of the joint

264 and its width. A good dressing and secondary cutting process produces stone elements

265 with the lateral faces perpendicular to the road surface, without marked indentations and

266 protrusions. There is a significant difference in the regularity of paving units of different

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267 varieties of stone and also from different quarries of the same variety. Some rocks can
268 be split up into suitable elements already after the quarrying and first cutting stage [34].
269 A special case is represented by cobbles; they are not dressed but naturally occurring
270 rounded and embedded, as found in river beds or beaches, with the smaller dimension
271 facing downwards. For blocks, slight undercut may be provided for the lower half of the
272 sides to allow the bedding material to find an outlet during the ramming and settling. In
273 the cubes, the taper (max. 1/8 of the upper face) can be extended to the whole depth
274 [38]. In this case, pavers are instructed to set the cubes with the larger face up; placing
275 the large end down, which is objectionable from a practical perspective, makes a too
276 wide joint. But, recent guidelines suggest to lay tapered cubes ensuring that adjacent
277 units are not in contact and the joint tolerances are maintained [9]; this recommendation
278 is actually not ever valid and excessively conditioned by the type of jointing product.
279 The side texture should have a suitable roughness able to optimize the adhesion and
280 frictional bonds with the jointing material: a too fine (sawn units) or too rough
281 (cropped) texture reduces the performance at the interface. For unbound jointing
282 material, the grading curve is selected on whether the stone unit is sawn or cropped, i.e.
283 finer in the first case and coarse in the second. In rigid construction, elements with split
284 or re-textured (sandblasted) surface generally show the greatest shear capacity, whereas
285 cropped elements present problems related to difficult in filling non-uniform joints,
286 resulting in variable performance under loading conditions [24].
287 Fairly smooth cobbles are set with the smaller dimension facing downwards upon a bed
288 of granular fine aggregate or natural soil and packed tightly together or bound with a
289 rigid medium. Although it is possible to lay cubes, setts and blocks using any jointing
290 material configurations, they are currently applied in rigid construction method, above
291 all in vehicular trafficked areas [39]. Unbound jointing on an unbound bedding,

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292 although presents some maintenance and functional drawbacks in presence of vehicular
293 traffic, represents the preferred methods for re-laying reclaimed elements from an
294 architectural conservation viewpoint. Traditionally, small deep elements (cubes and
295 setts) were split rather than cut, and the rough vertical faces were laid in dry-bound
296 manner almost in contact to each other, with a recommended joint width of 5÷10 mm
297 [40]. In general, ordinary stone-blocks pavements were laid so that any joint should
298 show a surface width of more than 10÷12 mm and a width of more than 22÷25 mm in
299 any part of the joint [12] (Tab. 1). In addition to be less costly, minimizing the amount
300 of required jointing material, keep the joint tight ensured on the one hand a better
301 vertical interlock, resulting in improved structural integrity of the pavement, and on the
302 other producing a reduction of the surface irregularity. Specifically, the joint width in
303 cubes and setts depends on element dimension ranging between 6 and 15 mm: 6÷10 mm
304 for small cubes (a = 40/60 mm); 8÷12 mm for medium cubes (a = 70/140 mm) and
305 shallower setts (c = 100/120); 10÷15 for large cubes (150/200) and deeper setts (c =
306 150/180) (Tab. 1), although achieving consistency in joint width is difficult in practice
307 above all for cropped or reclaimed units [17]. Only pavement on steep grades showed
308 comparatively wider joints to afford a good foothold for the horses [37]. But, today we
309 are witnessing a change in the stone pavement design, in which the installation of the
310 elements is subordinate to the types of jointing material so that it can develop its
311 potential. The joints have therefore become progressively wider (up to 25÷30 mm),
312 especially in cases of jointing with rigid or new generation products which require
313 larger joints width and depth than traditional products. In this way, besides
314 impoverishing the architectural and aesthetic quality of the pavement, is given to the
315 joint an extreme structural role (primary source of resistance to load) that could be

316 compromised at an early stage by local failures/fractures at the interface between the
317 jointing material and the stone or in the material itself (Fig. 8).



318
319 Figure 8 - Too wide joint width in a stone cube pavement

320 Ideally, flags should be bedded on a thoroughly compacted layer of unbound material
321 and jointed with the same product. This is the best solution for chamfered or rounded
322 flags. Although recommendations suggest the use of flexible construction methods for
323 shallow units, modern applications with calibrated flagstones and polygonal slabs are
324 realized using rigid joints, sometimes carried out as a single operation. However, this
325 approach remains problematic from both an aesthetic and technical point of view [29].
326 Hard impermeable joints are not able to absorb thermal and structural movements and
327 can cause erosion of stone edge due to the retention of moisture coupled with frost
328 action over time. The recommended joint width for flags and slabs is 6 mm to 12 mm;
329 but sawn stone shallow units manufactured with strict dimensional tolerances are
330 sometimes laid considering very narrow or butt joints (≈ 4 mm) (Tab. 1). When no joint
331 space is left between adjacent paving units, the stone is effectively “butted” together
332 and this can cause and favor spalling or delamination, exposing the edges of the slabs to
333 mechanical and atmospheric agents. With polygonal slabs, where it is not possible to

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334 speak of joints in a strict sense, the distance between the edges of two adjacent elements
335 should mainly be less than 30 mm.

336 **Laying patterns**

337 The choice of the most suitable pavement laying pattern is determined by aesthetic
338 requirements, but often dictated by functional ones, also in relation to the stone nature,
339 the route type, the adopted drainage system and the loading conditions [41]. The great
340 variability of dimension, shape and dressing of the stone elements promotes the
341 development of a wide range of pattern solutions, in which the joints and their networks
342 become themselves signs generating the pavement drawing [42]. The surface layout
343 uses geometric shapes, which can constitute a modular unit repeated according to a
344 certain rhythm in a non-directional or with prevailing orientation system [28]. This
345 incredible flexibility allows the laying pattern to be varied, passing from a linear to a
346 curved field, without increasing the joint width, which would be detrimental for the
347 uniformity effect and the serviceability of the surface. Thus, the designer has the
348 possibility to select different patterns that are able to solve, with solutions that enhance
349 the pavement aesthetic, even critical situations. In high stressed areas where horizontal
350 forces from braking, turning and accelerating vehicles are directly induced in the
351 surface (stop/start lines, junctions, steep inclines or marked changes of vertical gradient)
352 the use of specific patterns, even locally, is essential to generate horizontal interlock and
353 ensure the elements and joints to act homogeneously [43]. Whilst not always obvious in
354 the early life of a stone pavement, experience has shown that after a few months (time
355 depending on traffic volume) laying patterns that are not able to develop high levels of
356 horizontal interlocking can be disturbed by the traffic. As a rule, patterns that preclude
357 long straight lines in either direction and thus prevent movements developing under the
358 action of traffic are preferred [44]. Specifically, the size and the shape of the stone units

359 influence the selection of the bond pattern.

360 Regular cubes are generally laid in curved patterns, in which the arc is oriented in the
361 traffic flow direction. The arc is the most suitable construction element to horizontally
362 transmit and dissipate towards the lateral perimeter of the roadway the forces from
363 braking, turning and accelerating vehicles. This reduces joint compressive stress and the
364 potential for horizontal creep of units [45]. One of the most popular curved layouts is
365 the overlapping arc, also known as Bogen or segmental arc (Fig. 9a), in which the cubes
366 of each row are laid in a series of inter-laced arcs of constant radius and arranged in
367 such a way that each pair of neighbors arcs has in common the springer element [3]. In
368 this pattern, the central angle is equal to 90° and the length edge of the cubes gradually
369 increases from the springer to the keystone, i.e. the edge of the key element is equal to
370 the diagonal of the springer one. Since the face of the cubes is square and not
371 trapezoidal, the joints width grows, diverging at a certain angle, from the springer to the
372 keystone. But, too wide or irregular joints can compromise the aesthetic result and the
373 mechanical resistance of the paving, as well as reduce the users' comfort. Thus, each arc
374 is composed by about 20 cubes so that the angle of divergence does not exceed a few
375 degrees. Moreover, cubes must be placed staggered compared to those of the preceding
376 row to offer greater structural capacity and resistance to lateral movements. However,
377 the disadvantage of overlapping arc is that all the smaller elements (the less resistant)
378 tend to concentrate in narrow longitudinal areas (the springing lines), which are in a
379 parallel direction to the road axis, resulting in a potential weakness for the pavement
380 [46]. To overcome this drawback is possible to resort to the fan pattern, in which the
381 arcs are truncated, at points more or less distant from the springer line, to allow the
382 insertion of neighboring truncated arcs. The smaller cubes are no longer longitudinally
383 aligned but are arranged in a herringbone pattern along a strip of appropriate width and

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384 interspersed (spaced) with larger elements. The advantage does not just compensate for
385 the greater difficulty of installation: this layout is mainly used in areas where the main
386 purpose is to achieve a high decorative appearance. Another more valuable laying
387 pattern is the European fan or peacock tail of fantail one (Fig. 9b), in which cubes are
388 laid according to contiguous semicircles and setting the subsequent sectors on the center
389 of the already arranged semicircles [47]. A less effective alternative, able to generate
390 low to moderate interlock in the surface, is the coursed pattern in which the units are
391 laid in rows at 90° to the direction of traffic (transverse), eventually flanked by
392 longitudinal (running in same direction as traffic flow) channels or gutters at each edge,
393 but with each row staggered by approximately half a length. This represent the most
394 popular pattern chosen, since the XIX century, for sett work laid in streets and squares.
395 In some situations, the arrangement of the cubes diagonally to the direction of the traffic
396 (inclination of 45°), which represent a less effective solution, is widely used. The
397 advantage is the possibility of reversing the flow of traffic while leaving its resistance
398 unchanged [17].

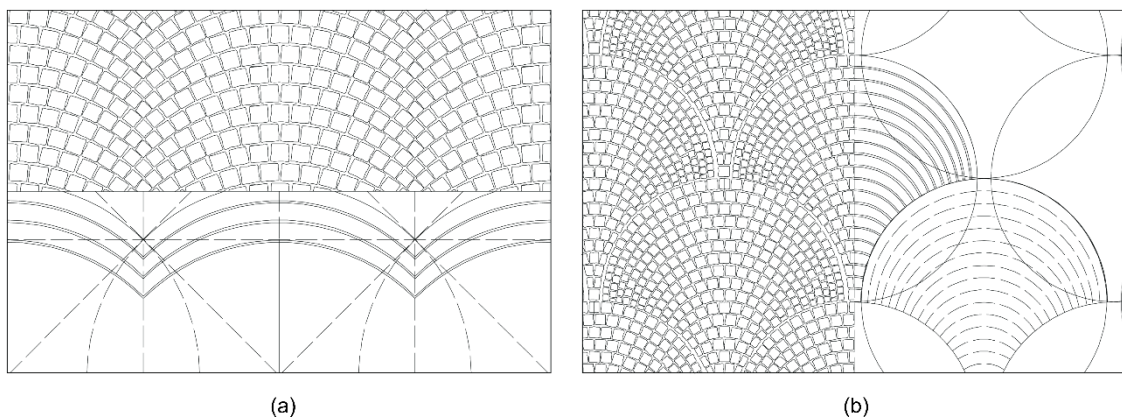


Figure 9 - Overlapping arc (a) and European fan (b) laying pattern

401 With regards to the elements with a rectangular plan, with different length to breadth
402 ratios (2:1, 3:1, 3:2 and 4:3 are the most common), the most effective laying pattern is
403 the herringbone bond, which offers greater structural capacity and resistance to lateral

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404 movements. In relation to the main direction or alignment of the pavement, stone
405 elements are placed either along and at right-angles (90°) or at about 45° , to this
406 direction [41]. Certain patterns assist also stone pavement realized with large flags or
407 blocks to withstand localized horizontal forces. In the past, the most used laying pattern
408 for vehicular traffic applications was that with courses inclined with respect to the road
409 axis. This arrangement was necessary to avoid that the metal rims traveled in the
410 directions of the joints, which in any case represent the weakest parts of the road
411 surface. These permanent stresses could have determined not only direct actions on the
412 edges but also displacements of the elements with consequent creation of tensions in
413 their perimeter surfaces. With respect to the road axis, the most suitable and used
414 inclination courses are those at 45° or 63° (Fig. 10). With this inclination, the edges of
415 the elements in correspondence to the connections are better saved from transit and the
416 number of head joints is minimized [11,40].

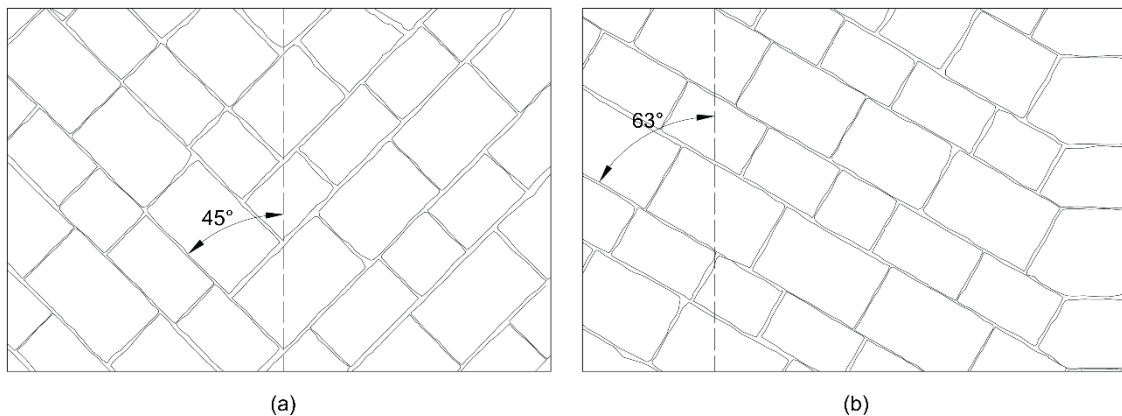


Figure 10 - Two methods of laying stone elements diagonally: 45° (a) and 63° (b)

419 Defects and pavement failure

420 Stone pavement defects, both in terms of performance and appearance, are caused by a
421 number of factors often acting simultaneously, which can be broadly classified into
422 three categories, i.e. mechanical, physical and chemical. These distresses can occur at
423 different severity level, ranging from slight deteriorations, which require small ordinary

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424 maintenance operations, to the complete pavement failure, which imply the elements'
425 lifting and relaying. Conceptually, flexible stone pavements typically fail under the
426 cumulative effect of a number of load cycles, whereas the rigid ones under a single
427 direct load, which can produce a shear failure of the joint and/or a bearing failure of the
428 laying course under the punching action [9,39]. Also lower severity levels that do not
429 compromise the pavement structural integrity, can reduce the serviceability,
430 representing also a source of discomfort or safety hazard for vulnerable users, and the
431 aesthetic rendering of the pattern [48,49]. Most of these defects are directly caused or
432 indirectly related to the alteration of the joint system, i.e. all the modifications that may
433 lead to the degradation of its original characteristics. Joint defects can be limited in
434 localized or isolated areas or more widespread. The former is very often attributable to a
435 concentration of stresses or a poor workmanship, while in the latter case is more likely
436 to be due to inaccurate design (Fig. 11).



437
438 Figure 11 - Stone pavement defects at stop/start line

439 Cracking (bound joint) or loss/pumping of jointing material (unbound joint) commonly
440 contribute to the loss of paving integrity. Two types of fractures can develop in almost

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441 all bound jointing materials, namely cracks along stone unit-filling contacts and
442 transverse fractures across the material [20]. However, fine surface cracks can appear
443 also in well-constructed and designed pavements and in most instances do not affect the
444 load transfer capability of the surfacing [24]. But, when cracking is associated to the
445 lack of punching shear resistance, a rapid deterioration induced by the vehicle tires can
446 spread to the bedding layer [21]. In parallel, cracked joints become preferred paths for
447 water, salt and granular debris such as freeze-thaw cycles, dissolution, salt
448 crystallization, and abrasion will occur within the crack, leading to an ever greater
449 severity level (progressive cracking), up to the material breakdown its removal from the
450 joints [39]. Similarly, for unbound joints the emptying of the joint can be caused by the
451 wear of traffic or surface cleaning operations, poor management and disposal of
452 moisture accumulation (rainwater and runoff) and pumping under traffic loading [25].
453 Once the homogeneity of the surface matrix is broken, the structural benefit guaranteed
454 by the bond adhesion strength and/or interlocking is lost and units can start to move
455 independently of their neighbors. Inevitably, these phenomena increase inconsistently
456 the joint width, above limit values necessary to control lateral movements and stabilize
457 the elements which can be easily removed (small cubes or setts). Any subsequent
458 dynamic or torsional force on the unconstrained elements lead to rototranslation
459 mechanisms in both vertical and horizontal planes, resulting in faulting, i.e. difference
460 or rotation of the adjacent units' elevation, punching, i.e. vertical downward movements
461 in and around wheel paths, and horizontal creep, i.e. longitudinal displacements of the
462 stone elements and pavement (Fig. 12) [46,50].



463
464 **Figure 12 – Stone pavements affected by faulting, punching, horizontal creep and asphalt**
465 **patching**

466 Besides, the joint emptying exposes the edges of the elements to traffic and weather
467 conditions favoring stone chipping, delamination and rocking above all for large and
468 shallow elements. Moreover, extremely wide and empty joints become receptacle of
469 urban dirt and litter. In these case it will necessary the joint repair, through refilling
470 operations. Great care is required above all in rigid construction, in which the joint
471 should be raked out and cleaned before the application of the new product. Poorly
472 matched materials following these interventions in localized areas may also be regarded
473 as a defect at an aesthetic level [29]. In addition, the prolonged, continuous and/or
474 frequent exposure of the pavement to atmospheric agents (moisture, temperature
475 variation and UV radiation) or chemical pollutants can cause a chromatic alteration of
476 the stone elements and jointing material or give rise to the formation of surface deposits
477 due to the progressive accumulation of dirt, biological coatings (molds, weeds, mosses
478 and lichens) and salt efflorescence [51,52].

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479 **Jointing materials' selection and application technique**

480 Downstream of the decision-making process on the characteristics and requirements
481 that the joint must satisfy, a synthesis is reached with the identification of the most
482 coherent material (Tab. 2). The currently used materials and their application techniques
483 are not very different from those of the past, even if the jointing products industry is
484 constantly evolving to satisfy the demand for efficient and durable materials
485 characterized by a fast, clean and simple installing method [16,45]. In general, unbound
486 products are dry brush-in applied, while the bound materials according to their
487 consistency (liquid, slurry, viscous or pasty) can be poured, injected or hand pointed.
488 Each jointing medium **works** best when installed over a pavement characterized by
489 proper moisture conditions (dry, slight or full wet) and may require specific equipment
490 and tools and/or skilled workmanship (Tab. 3).
491 As a rule, unbound or flexible joints are realized over an unbound bedding layer
492 (permeable system) of dry and sharp crushed aggregate of graded particle size.
493 Traditionally, joints in pavements were filled with unbound aggregates [44]. Fine (**sawn**
494 **elements and narrow joints**) or coarse (**cropped units and wide joints**) and sharp sand,
495 capable to develop mechanical interlock and friction for stabilizing the stone elements,
496 is uniformly spread over the completely dried surface and swept with a push broom into
497 the joints, down to their full depth, before being compacted using a vibrating plate or a
498 hand tamper. Inconsistent or inadequate compaction will lead to early failure of the
499 pavement. However, vibration is most commonly employed with smaller and deeper
500 elements and less often with larger and shallower units. Sand jointing can be carried out
501 simultaneously with vibration, alternating compaction and filling the joints until they
502 are completely saturated. Eventually, these operations can also be executed on a soaked
503 pavement to aid the settle of the material. Finer dust (0/3 mm) can be finally added,

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504 especially using deeper stone element, to a thickness between 5 to 10 mm. When dust is
505 applied, the surface can be wetted with a water spray to wash it into the joints. Unbound
506 jointing, in addition to be cheap and easy to install, creates a permeable pavement
507 characterized by a good flexural strength and improved aesthetic (a wide range of colors
508 is available in naturally-occurring aggregates). The absence of curing period allows an
509 immediate opening of road or footpath to traffic. Moreover, the pavement may be lifted
510 and re-laid without the breakage of the stone elements [25]. This technique, although
511 even today represents the most cost-effective jointing solution for pedestrian areas,
512 lightly trafficked zones and perfectly draining pavements [53], did not well fit with the
513 demands of the today's infrastructures which require high level of load carrying
514 capacity. Sand does not protect the edge of the units from chipping and may be washed
515 out on steep slopes or in areas subjected to frequent surface water flows or on
516 pavements subjected to vacuum/brush street cleaning sweepers. To overcome the latter
517 drawback, a joint stabilizing product, a paving sealant or a topping of limestone dust
518 should be applied to protect against the loss of material. Moreover, top-up of joints with
519 additional material may be required in pavement early life as the system 'shakes-down'
520 and 'stiffens up' under traffic loading [23]. Among the dry brush-in type alternatives
521 should be included the sand-cement mixtures (dry-grouting) and two different families
522 of products, i.e. polymeric sands and the polymeric resin mortars, which are recently
523 appeared on the market. These products have been designed to retain adequate elasticity
524 needed to adapt to the movements of the pavement surface, preventing at the same time
525 erosion due to weather (wind, rain and freeze thaw cycles) and light traffic. Polymeric
526 sands and resin mortars are high-tech mixtures of graded and calibrated fine sand
527 combined with polymer additives, which are able when activated (water and/or air
528 oxygen) to form a strong binding agent capable of locking the sand particles together

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529 [54,55]. These products follow the same installation procedure used for the sand, but
530 require two additional stages to complete the catalyst process: wetting and drying. The
531 activation of the polymeric binding agent is achieved by spraying, from 1 to 3 times, the
532 paver surface with a very fine mist, whereas dry-grouting rely on natural moisture to
533 cure and hydrate the cement. Most recent polymeric resin mortars work best when
534 mixed on site with plenty of water, becoming of a slurry consistency, and spread on the
535 pre-wetted pavement using a squeegee. Polymeric products need to dry completely
536 (curing time: 24-72 hours) to polymerize and harden, ensuring optimal cohesion and
537 long term stability. Air oxygen hardening polymeric resin mortar has few minutes of
538 working life and requires 6-8 hours to complete the curing process. For both water-
539 activated products, pedestrian and vehicle traffic should be kept off the pavement for at
540 least 24 and 48-72 hours from the installation. Polymeric sand and mortar, like regular
541 sand, come in a wide array of colors, representing viable options for jointing private
542 drives, paths, patio and hard landscaping projects. But, although the market for these
543 products is strongly growing, their properties and potential have been poorly studied by
544 the scientific literature. The use of sand-cement mix, which unlike other products
545 requires a dry or semi-dry bedding, is not actually recommended, as the cement does
546 not fully hydrate and the mixture does not compact in a consistent manner the joint will
547 ensure poor durability and adhesion properties.

548 In parallel, different types of bound jointing material are in common use, responding in
549 a flexible or rigid manner. Coal tar pitch or asphalt-based products became a viable and
550 affordable option in the late-XIX century as a flexible joint medium [36]. However, coal
551 tar, which is considered a potentially hazardous material and is now classified as
552 carcinogenic, has not been used by at least 30-40 years. For tight joints, asphalt cement
553 alone is used; for wider joints a mastic (asphalt cement + fine sand + filler) is preferred.

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554 Another option with open joints is to pour the hot (160-200 °C) asphalt cement
555 (penetration grade depending on the latitude and the exposition) over the partially filled,
556 by pre-heated gravel or pebbles (6/10 mm) to within 20÷25 mm of the head of the stone
557 elements [25]. It is essential that the filling aggregates and the stone elements are
558 completely dry because an almost unappreciated amount of water will cause the asphalt
559 cement to foam and will prevent it from adhering and forming a solid joint. Asphalt
560 cement is spilled in several stages until complete saturation from a pouring bucket or
561 can and allowed to flow, making the pavement impervious as it cools. Alternatively,
562 asphalt emulsion (asphalt cement + water), which is cold applied into the joints
563 previously saturated with gravel, can be used [15]. A final cleaning with fine sand can
564 be envisaged. Asphalt binder cools and **solidifies** within few minutes, although it will
565 remain warm to touch and relatively soft for some hours, period after which the
566 pavement can be opened to traffic. The success or failure of the asphalt filling depends
567 on the product efficiency and the heating temperature. If the binder be too hard, it
568 pulverizes in very cold weather; if it be too soft, it runs and becomes sticky in very hot
569 weather. Any accidental spillage or overflow from the grout lines can be trimmed with a
570 craft knife after the asphalt cement has cooled and hardened. Black traces of binder and
571 residual staining on the surface will be removed only due to weathering and tire actions
572 over time. Nowadays, this approach has a potential on repairs to authentic historic
573 paving, the so-called “heritage” projects, using specifically tailored modified asphalt
574 cements. Although freshly-poured asphalt cement could tend to have a glossy
575 appearance, the joints become dull over few days as the surface is trafficked, retaining
576 permanently their black deep color [51]. A technically valid alternative, effectively
577 tested to date only as binder in asphalt wearing course or surface treatments, would be
578 represented by clear binders (bio-binders and synthetic binders), eventually also in the

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579 form of emulsion, or products with similar mechanical, rheological and physical
580 performance compared to the traditional asphalt cements but characterized by a pale
581 amber color. These materials would allow to customize the joint color palettes,
582 exploiting the chromatic properties of stone aggregates or pigment powders [31-33].
583 Since the early 2000s, the use of a new technology, i.e. the **semi-rigid** resin system, that
584 presents application similarities to asphalt-based jointing is progressively establishing in
585 installations for the restoration of pre-existing road pavements deteriorated by traffic
586 realized in cubes, destined or restricted to local public transport. Although such solution
587 is even prescribed in high demanding operation class by some very recent standards, it
588 is premature to define its reliability, durability and maintenance of performance over
589 time because it has not yet been tested over a prolonged period of time under different
590 stress conditions. This system involves the use of a resin (one-component liquid amber
591 mixtures of polyurethane or epoxy polymers) which is poured over a pre-filled
592 aggregate joint to form a pervious and strong structure. Unlike asphalt-based product,
593 resin requires a further filling of finer rock grit (2/4 mm) for the whole joint depth after
594 a second vibro-compaction phase. The resin is then hand poured at ambient temperature
595 directly into the joint using a low-pressure injector/nebulizer. The time required for the
596 resin to completely catalyze is at least 36 hours; thus, the pavement can be opened to
597 traffic in a short time, within 48 hours of its application, under normal temperature
598 conditions. The joint will appear, even after the time required for complete drying, of
599 the aggregate color with a slightly darker shade and a glossy finishing due to the amber
600 thin film of the resin. However, resin overfilling, which can be prevented only by the
601 operator skills and experience, leads to the complete loss of aesthetic requirements. In
602 addition, any accidental spillage or residue that deposit on stones during joint grouting,

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603 which result in amber-colored and glossy droplets, should be removed before they
604 become permanent and treated with a suitable cleaner or thinner.

605 As far as rigid jointing is concerned, hydraulically bound mortars and grouts become
606 the most common method, as the motor traffic grew and with the development of
607 reliable cements [20,21]. They are workable mixtures of sand, with a suitable grain size,
608 mixed with cement and water with a variable proportion. This classification includes
609 different forms of cementitious binders: ordinary or blast furnace Portland cement and
610 lime. Originally, mixed on site products were prepared, while the most recent
611 applications push towards the use of pre-mixed and modified mortars or grouts with
612 enhanced properties (better strength, adhesion or elasticity), the so-called proprietary
613 mixes [56]. Latex, redispersible dry and water-soluble polymers are the different types
614 of additives used to produce polymer modified mortars [57,58]. Cement-based mixtures
615 must achieve the right balance between strength (compressive, flexural, shear), low
616 shrinkage and durability (resistance to frost and to de-icing salts). In general, higher
617 performance mortars can be used with smaller setts or cubes; lower grade mortars with
618 larger setts and slabs [24]. The application method is dependent on the jointing material
619 consistency and the bed layer typology. Although some mortars are applied using the
620 traditional hand-pointing methods, the two most popular techniques are joint topping
621 (lightly loaded areas) and full depth (high loading category) applied on moist mix bed
622 and slurry joint with a plastic bed. In hand-pointing, the mortar is firmly pressed into the
623 joints with a trowel or a suitable rod: flush-and-struck, recessed and half-round are the
624 most suitable pointing solutions. These mortars generally start setting within a couple of
625 hours (high temperatures reduce and low temperatures increase the setting time) and the
626 pavement should be pedestrian and vehicular traffic-restricted for at least 48 and 120
627 hours, respectively. Moist bed with topping joint require a three-stage process. Firstly,

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628 the joints are about halfway filled from the bottom by the rising of the laying course
629 material. Additional cement mortar is put into the joint from the top and the pavement is
630 compacted using a plate vibrator. The joints are then raked back to a depth of 30mm
631 before re-grouting with a fine concrete slurry. In full depth applications, the moist or
632 plastic laying course, which should not rise more than 15-20 mm up the joint, is
633 allowed to cure to reach initial set before the joint filling operation. After about 24
634 hours, flowable self-compacting grouts are spread over the thoroughly pre-wetted
635 pavement surface and moved with a rubber squeegee towards open joints (wet-grouting
636 or slurry method), whereas more consistent modified mortars are poured through a
637 watering can or an injection gun into the empty joints. One traditional method, now
638 almost abandoned because can result in a loss of durability of joint mortar, was to coat
639 the surface with sawdust and brush this off. After completed operations, any excess of
640 material must be scraped off using water jet and swept with a bristle brush, because the
641 persistence of hardened cement stains and glazes significantly compromises the
642 aesthetic result of the entire pavement and their removal is very difficult, if not
643 impossible. The curing time determines when the pavement can be subjected to traffic
644 loading. Traffic must be kept off until the mortar or grout sets and this is necessary not
645 only when the pavement is first laid, but whenever plumber's cut or worn spot is
646 repaired. Normally, cement-based products achieve the specified characteristic strength
647 after 28 days (2/3rds after 14 days). If quicker pavement opening to traffic is required,
648 higher strength mortar or accelerating additives might be used. The potential of ultra-
649 rapid-hardening mortar for jointing stone small element have been recently investigated
650 [39]. The hydraulic-cement mortar and grout make the joint impervious, holds the stone
651 elements firmly in position, prevent the edges from chipping and the top face from
652 wearing round and adds materially to the smoothness and durability of the pavement.

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653 However, there are other objectionable features connected with these family of
654 products. Firstly, a single shear stress application in excess of the material capacity
655 cause a local failure and it wears down of breaks out from between the elements it
656 cannot be renewed or added to with ease and success. In making repairs, which resulted
657 to be time consuming and expensive, it is difficult to remove the stone elements without
658 braking and to remove the hardened jointing material from the sides of the stone units.
659 If the mortar does not penetrate to the bottom of the joints, bad defects result as the
660 thrust from expansion then as to be carried by the upper part of the joint only, and this
661 crushes the heads of the elements. For most site-mixed mortars the end color is
662 determined by the color of the sand, but standard mortar can be colored by the addition
663 of liquid or powder pigments. Alternatively, pre-packed colored mortars can be
664 obtained.
665 An overall summary of the properties and performances of the above mentioned
666 jointing materials and their related application characteristics is reported in tables 2 and
667 3.

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8 668 Table 2 – Properties and performances of jointing materials
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Type	Behavior	Load transfer	Physical state	Drainability	Appearance		
					Color	Finish	Brightness
Fine sand/Gravel	Flexible	Friction	Granular solid	Good	Aggregate color	Fairly grainy/Grainy	Matte
Polymeric sand	Flexible	Friction/Adhesion	Granular solid	Poor	Sand color + pigment	Fairly grainy	Matte
Polymeric resin mortar	Flexible	Friction/Adhesion	Granular solid	Poor	Sand color + pigment	Fairly grainy	Matte
Asphalt base compound	Cement	Flexible	Solid	None	Black/dark grey	Smooth	Matte
			Viscous (80-160 °C)				
	Mastic	Flexible	Adhesion	None	Black/dark grey	Smooth	Matte
			Liquid (> 180°C)				
			Viscous (80-160 °C)				
Emulsion	Flexible		Liquid		Smooth	Matte	
Mortar	Rigid		Pasty/self-compacting		Grey + pigment	Smooth	Matte
Cement based	Modified mortar	Rigid	Adhesion	Poor	Grey + pigment	Smooth	Matte
	Grout	Rigid			Grey + pigment	Smooth	Matte
Polyurethane resin	Semi-rigid	Friction/Adhesion	Liquid	Quite good	Gravel color + thin amber film	Grainy	Glossy

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670 Table 3 – Application characteristics and maintenance program of jointing materials

Type	Application			Operator		Opening to traffic		Cost	Frequency ordinary maintenance	
	Technique	Filling time*	Main tools	Working position	Skills	pedestrian	vehicular			
Fine Sand/Gravel	Dry brush-in	Short	Sweeping broom	Standing	Low	Immediate		Low	High	
Polymeric sand	Dry brush-in	Short	Sweeping broom + Garden hose	Standing	Low	24 h	48/72 h	High	Non-documented	
Polymeric resin mortar	Dry brush-in	Short		Standing	Low	24 h	48/72 h	High	Non-documented	
	Wash-in	Short	Squeegee + Garden hose	Standing				High		
Asphalt base compound	Cement			Kneeling	High			Medium	Low	
	Mastic	Pouring	Long	Pouring bucket	Kneeling	High	12 h	24 h	Medium	
	Emulsion			Kneeling	High			Medium		
		Pouring	Short	Squeegee	Standing	Low			Low	
	Mortar	Injection	Short/Medium	Caulking gun	Kneeling	High			Medium	
Cement based		Hand-pointing	Medium/Long	Trowel	Kneeling	Medium	48 h	7/10 days	Low	Low (early life)
	Modified mortar	Pouring	Short	Squeegee	Standing	Low	(min.)	(min.)	Medium	High (after cracking)
		Injection	Medium/Long	Caulking gun	Kneeling	Medium			Medium	
	Grout	Slurry	Short	Squeegee	Standing	Low			Low	
Polyurethane resin	Injection	Long	Sweeping broom+ injector/nebulizer	Kneeling	High	12/24 h	24/48 h	Very high	Non-documented	

671 * excluding curing time

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672 **Conclusions**

673 Natural stone paving can produce a high quality attractive streetscape with a long
674 service life. The beauty and excellent performances of natural stone are necessary but
675 not sufficient conditions to fully satisfy the users' needs and to enhance the aesthetic
676 value of the pavement. Only correct design, careful installation and prompt maintenance
677 of all components ensure the pavement the basic structural and functional requirements.
678 However, these principles of good construction practice with reference to the joints
679 have been and still are wrongly neglected and understudied. Thus, after having
680 examined several tasks performed by the joints, factors affecting their performances
681 and, at the same time, how their characteristics influence the pavement behavior and
682 appearance have been analyzed in detail. Specifically, particular attention has been paid
683 to the design phase. The decision-making process that leads the designer to choose the
684 more suitable joint system represents the synthesis of a complex procedure that requires
685 the necessary assessment of multiple criteria of aesthetic, functional and structural but
686 also historical and cultural. This extremely complex selection mechanism is made
687 further difficult by the stone pavement specific intended use and, above all, the
688 environmental and architectural context in which it exists or is to be constructed. The
689 recent renewed interest in stone pavements has often led to the implementation of
690 restoration and conservation interventions of historical streets and squares in
691 monumental areas of high intrinsic value. In these situations, the best theoretically
692 technical jointing solution cannot exceed the noblest requirements regarding the
693 preservation of the cultural significance of the road work, the recognizability of the
694 aesthetic and the maintenance of the hygienic and functional characteristics typical of
695 epochs prior the motorization development. The analysis of the logical path for the joint
696 design must be based on a specific study, sometimes necessarily respectful of

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697 construction rules contained in centuries-old specifications, to achieve only at the end
698 the identification of the most suitable material and the related laying technique. The
699 most modern trends and state of practice tend instead to subvert the necessary process
700 of engineering analysis and construction art, with the paradox that often the jointing
701 material, its width and mode of filling become the starting point of the project,
702 conditioning the type of laying pattern of the stone elements, their mutual arrangement
703 and, finally, also the interlocking for the horizontal and vertical transmission of stresses
704 in the pavement.

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DETAILED RESPONSE TO REVIEWERS

Parma, 14th May 2020

Dear Editor-in-Chief,

We have received your letter indicating that a revised version of our manuscript “CRITERIA FOR THE SELECTION AND DESIGN OF JOINTS FOR STREET PAVEMENTS IN NATURAL STONE” (CONBUILDMAT-D-20-02803) was requested. We strove to provide you a new better revised article following closely all the reviewers’ comments, which were particularly useful for identifying aspects not well clarified and for meeting Journal standards. From that perspective, we would like to thank them for their valid opinions and time spent.

In the following lines we detail for each paper section the changes in accordance to all reviewer’s comments, suggestions and questions. To improve reading, all the new or revised parts in the submitted revised manuscript have been identified using red characters.

GRAPHICAL ABSTRACT

In accordance to **reviewer 2** advice, a graphical abstract has been added to offer a clear and concise overview of the paper since the beginning.

DESIGN CRITERIA

As suggested by **reviewer 1** and **reviewer 2**, a flow chart has been inserted to clarify the multi-criteria analysis and the decision making process.

According to **reviewer 1** recommendations, Figure 3 (now Figure 4) has been redrawn to highlight the difference between action and reaction forces. Besides, each part of figures 2, 3 and 4 (now 3,4 and 5) was labelled and cited in the text.

FEATURES OF ELEMENTS

Upon the suggestions of **reviewer 1**, the individual characteristics and functions of each stone element have been summarized in a single table (new Table 1).

The sentence: "*relationship between progressive wider joints and cases of jointing with rigid or new generation products which are characterized by minimum joint width and depth requirements*", has been rephrased. (**reviewer 2**)

DEFECTS AND PAVEMENT FAILURE

Upon the suggestions of **reviewer 1** and **reviewer 2**, a multiple figure (new Figure 12) representing examples of stone pavements affected by several distresses (faulting, punching, horizontal creep and asphalt patching) has been added.

JOINTING MATERIALS' SELECTION AND APPLICATION TECHNIQUE

According to **reviewer 1** recommendations, the paragraph title was changed and the Table 1 (now Table 2) and Table 2 (now Table 3) are now referenced in the text also before the last paragraph.

To conclude, all the minor changes, such as typos errors or clarifications, highlighted by the **reviewer 2** have been corrected or/and inserted in the text.

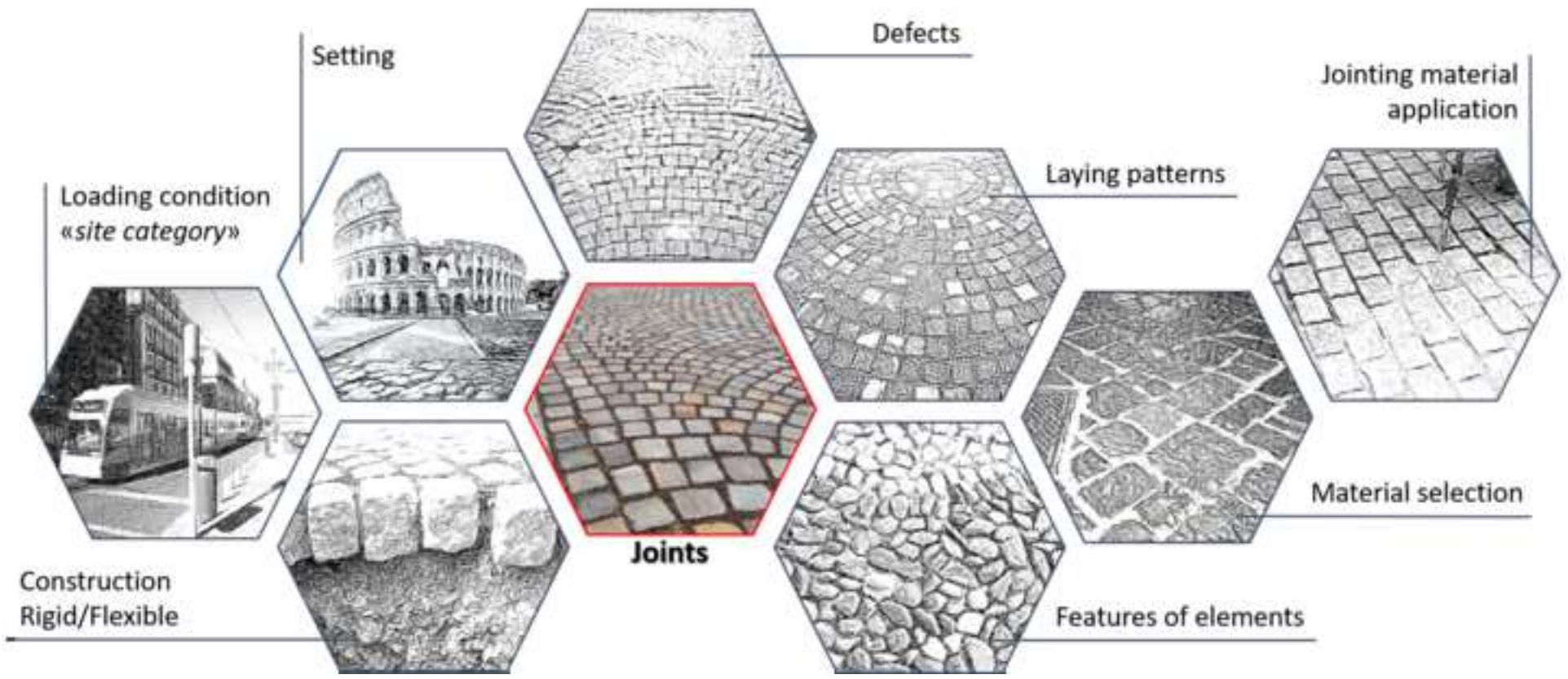
We are aware of the opportunities you offered us to improve the paper, such as detailing aspects not well explained in the previous draft. Thank you again for your contribution in improving this paper and for your constructive feedback.

Sincerely,

Felice Giuliani

Highlights

- A multi-criteria analysis for the design of joints in natural stone pavements was proposed.
- Functional and aesthetic hierarchical requirements of joints were identified.
- Several aspects which contribute to an optimized joint performance were analyzed.
- A jointing materials' state of practice accompanied by their application techniques was reported.



Declaration of interests

The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

The authors declare the following financial interests/personal relationships which may be considered as potential competing interests:

Title

Criteria for the selection and design of joints for street pavements in natural stone

Author names and affiliations

Federico Autelitano^a, Erika Garilli^a and Felice Giuliani^{a*}

^a*Dipartimento di Ingegneria e Architettura, University of Parma, Parma, Italy.*

* Dipartimento di Ingegneria e Architettura, University of Parma, Parco Area delle Scienze,
181/A - 43124 Parma, Italy. E-mail: felice.giuliani@unipr.it Phone :+39 0521 905905

Corresponding author

Felice Giuliani

E-mail: felice.giuliani@unipr.it Phone :+39 0521 905905

Dipartimento di Ingegneria e Architettura, University of Parma, Parco Area delle Scienze,
181/A - 43124 Parma, Italy